Ideal biological reactors (*a simplified steady state model for the activated sludge unit*)

In a biological system for wastewater treatment, organic pollutants or nutrients (N and P) are removed from the water streams and used by microorganisms that develop within the system. Main reactions are biochemical ones.

They are widely applied in environmental engineering:

- Aerobic degradation of organic pollutants
- Biological Nitrogen and Phosphorous removal
- Anaerobic degradation of organic pollutants from wastes and wastewaters (waste sludge digestion, biogas production from agro-wastes, OFMSW stabilization)

•Composting systems

•Biofilters for volatile organic compounds removal from gaseous flows.

•Bioremediations of contaminated soils

 \bullet ...

Many biological systems include a solid/liquid separation unit that allows for the selective retention of microorganisms within the system. As a matter of fact, the higher their concentration, the faster the degradation process will proceed. Widely employed separators are:

- •Gravity settling unit
- •Flotation units
- •Filters/membranes
- •Supports for the development of a microbial biofilm.

By retaining particulates, the following residence times can be differentiated:

- •*The hydraulic retention time (θ^H)* that is the average residence time of water and of all soluble (unretained) substrates
- •*Biomass retention time (θ^C)* that is the average residence time of all particulate substances that are efficiently retained in the biological system.

Systems with/without biomass retention can be described as follows:

Setting **MASS BALANCES** across biological reactors in flow *Hydrodynamics*: Continuous Flow Stirred Tank Reactor, CFSTR, with constant volume (V)

Lets consider the case of a retained (X) or unretained (S) component, at steady state: *d*[S

 $$ *X C* $\frac{C}{\rho}$ $\frac{VH \cdot [N]_0 - [N]}{N} + r_X = 0$ *X H C* $\frac{0}{2} - \frac{N}{2} + r_{x} = 0$ $Q \cdot [X]_0 - Q_X \cdot [X] + V \cdot r_S = 0$ \mathcal{W} where $\colon \theta_H = \mathsf{V} \mathsf{/} \mathsf{Q}$ *S H* $r_S = 0$ $Q([S]_0 - [S]) + V \cdot r_S = 0$ $X]_0 - [X]$ *mat*
or : $X]_0$ $[X]$ $V \cdot \frac{dV}{dt} = Q \cdot [X]_0 - Q_X \cdot [X] + V \cdot$
that can be rewritten as : *d t d*[X $V \cdot \frac{q[X]}{I} = Q \cdot [X]_0 - Q_X \cdot [X]$ $[S]_0 - [S]$ $V \frac{dV - L}{dt} = Q([S]_0 - [S]) + V \cdot r_S = 0$
that can be rewritten as : *d t* $V \frac{d[S]}{L} = Q([S]_0 - [S])$ $[X]_0 - [X]$ $[X]_0$ $[X]$ $[X]$ $[S]_0 - [S]$

 r_i = overall reaction rate \rightarrow From Petersen matrix

•Biomass retention is especially interesting when biomass growth is the kinetically limiting process; biomass wash-out can be easily prevented by adopting adequate θ_c without over-sizing the biological reactor

• Since biomass does the job, the higher its concentration the faster is the degradation per unit of volume, the smaller the biological reactor.

• Biomass maximum in-reactor concentration depends on the efficiency of the retention unit or by other operational parameter (e.g. oxygenation)

Example: Let us considered a biological system for the aerobic degradation of organic pollutants.

Data:

- wastewater characteristics: [X_S]₀, [S]₀
- Reactor hydrodynamics: CFSTR with biomass retention
- Petersen Matrix:

$$
r_1 = \hat{\mu} \cdot \left(\frac{S}{S + k_S}\right) \cdot X_B \qquad r_2 = k_H \cdot \left(\frac{X_S/X_B}{X_S/X_B + k_X}\right) \cdot X_B \qquad r_3 = b \cdot X_B
$$

$$
r_1 = \hat{\mu} \cdot \left(\frac{S}{S + k_S}\right) \cdot X_B \qquad \qquad r_2 = k_H \cdot \left(\frac{X_S/X_B}{X_S/X_B + k_X}\right) \cdot X_B \qquad \qquad r_3 = b \cdot X_B
$$

These are the relevant mass balances at steady state:

$$
\frac{d[S]}{dt} = \frac{([S]_0 - [S])}{\theta_H} + \left[-\frac{\hat{\mu}}{Y} \cdot \left(\frac{[S]}{[S] + k_S} \right) \cdot [X_B] + k_H \cdot \left(\frac{[X_S]/[X_B]}{[X_S]/[X_B] + k_X} \right) \cdot [X_B] \right] = 0
$$
\n
$$
\frac{d[X_S]}{dt} = \frac{(\theta_C/\theta_H \cdot [X_S]_0 - [X_S])}{\theta_C} + \left[-k_H \cdot \left(\frac{[X_S]/[X_B]}{[X_S]/[X_B] + k_X} \right) \cdot [X_B] + (1 - f) \cdot b \cdot [X_B] \right] = 0
$$
\n
$$
\frac{d[X_B]}{dt} = \frac{(0 - [X_B])}{\theta_C} + \left[\hat{\mu} \cdot \left(\frac{[S]}{[S] + k_S} \right) \cdot [X_B] - b \cdot [X_B] \right] = 0
$$
\n
$$
\frac{d[X_P]}{dt} = \frac{(0 - [X_P])}{\theta_C} + \left[\hat{\mu} \cdot b \cdot [X_B] \right] = 0
$$

This is a non-linear system that can be solved numerically.

To get an analytical simple solution, a simplified case can be usefully considered by neglecting the hydrolysis process thus considering that the influent wastewater contains biodegradable/soluble organics,

unbiodegradable/soluble organics, unbiodegradable/particulate organics.

$$
r_1 = \hat{\mu} \cdot \left(\frac{S}{S + k_S}\right) \cdot X_B
$$

$$
r_3 = b \cdot X_B
$$

The system simplifies at steady state:

$$
\begin{bmatrix} \neq 0.1: \frac{d[S]}{dt} = \frac{([S]_0 - [S])}{\theta_H} + \left[-\frac{\hat{\mu}}{Y} \cdot \left(\frac{[S]}{[S] + k_S} \right) \cdot [X_B] \right] = 0 \\ \neq 0.2: \frac{d[X_B]}{dt} = \frac{(0 - [X_B])}{\theta_C} + \left[\hat{\mu} \cdot \left(\frac{[S]}{[S] + k_S} \right) \cdot [X_B] - b \cdot [X_B] \right] = 0 \\ \neq 0.3: \frac{d[X_P]}{dt} = \frac{(0 - [X_P])}{\theta_C} + \left[\left[\cdot b \cdot [X_B] \right] \right] = 0 \end{bmatrix}
$$

This system is easily solved:

eq. 2:
$$
\frac{[X_B]}{\theta_c} = \left[\hat{\mu} \cdot \left(\frac{[S]}{[S]+k_s}\right) \cdot [X_B] - b \cdot [X_B]\right] \text{ by simplifying } [X_B]:
$$
\n
$$
\frac{1}{\theta_c} = \left[\hat{\mu} \cdot \left(\frac{[S]}{[S]+k_s}\right) - b\right], \text{ solving for [S]:}
$$
\n
$$
[S] = \frac{k_s \cdot (1+b \cdot \theta_c)}{\theta_c \cdot (\hat{\mu} - b) - 1}
$$
\neq. 1:
$$
\frac{([S]_0 - [S])}{\theta_H} = \left[\frac{\hat{\mu}}{Y} \cdot \left(\frac{[S]}{[S]+k_s}\right) \cdot [X_B]\right] = 0
$$
\nremebering that, from eq. (1):
$$
\hat{\mu} \cdot \left(\frac{[S]}{[S]+k_s}\right) = \frac{1}{\theta_c} - b \text{ substituting :}
$$
\n
$$
\frac{([S]_0 - [S])}{\theta_H} = \frac{1}{Y} \cdot \left(\frac{1}{\theta_c} - b\right) \cdot [X_B] \text{ solving for } [X_B]
$$
\n
$$
[X_B] = \frac{\theta_c}{\theta_H} \cdot \frac{Y \cdot ([S]_0 - [S])}{1+b \cdot \theta_c} = \frac{\theta_c}{1+b \cdot \theta_c} \cdot \frac{Y \cdot ([S]_0 - [S])}{\theta_H}
$$

eq. 3:
$$
\frac{[X_{P}]}{\theta_{C}} = \left[\cdot b \cdot [X_{B}]\right]
$$

$$
[X_{P}] = f \cdot b \cdot \theta_{C} \cdot [X_{B}]
$$

In summary, the following expressions have been found

$$
[S] = \frac{k_S \cdot (1 + b \cdot \theta_C)}{\theta_C \cdot (\hat{\mu} - b) - 1}
$$

$$
[X_B] = \frac{\theta_C}{\theta_H} \cdot \frac{Y \cdot ([S]_0 - [S])}{1 + b \cdot \theta_C}
$$

$$
[X_P] = f \cdot b \cdot \theta_C \cdot [X_B]
$$

$$
[\mathcal{S}] = \frac{k_{\mathcal{S}} \cdot (1 + b \cdot \theta_{\mathcal{C}})}{\theta_{\mathcal{C}} \cdot (\hat{\mu} - b) - 1}
$$

The concentration of degradable substrate that is left at the end of the process is affected by: •Kinetic parameters of biomass (kS and b) •The biomass retention time

• It does not depend on $[S]_0$

$$
[X_B] = \frac{\theta_C}{1 + b \cdot \theta_C} \cdot \frac{Y \cdot ([S]_0 - [S])}{\theta_H}
$$

- The biomass concentration in the system grows :
- •with the treated organic load
- \cdot with θ_{C} , up to an asymptotic value

 $[X_P] = f \cdot b \cdot \theta_C \cdot [X_B]$

Biomass debris grows :

- •with the decay rate
- •with biomass concentration and thus with the treated load •with θ_{C}

The total amount of soluble and particulate organics in the system includes the inert fractions: *[S_i]_o, [X_i]_o*. These components do not react, therefore their mass balance is simply:

eq. 4:
$$
\frac{d[S_i]}{dt} = \frac{([S_i]_0 - [S_i])}{\theta_H} = 0
$$

eq. 5:
$$
\frac{d[X_i]}{dt} = \frac{(\theta_C/\theta_H \cdot [X_i]_0 - [X_i])}{\theta_C} = 0
$$

therefore :

$$
[S_i] = [S_i]_0
$$

$$
[X_i] = [X_i]_0 \cdot \frac{\theta_C}{\theta_H}
$$

*Total soluble and particulate compound*s are:

$$
[S_{\tau}] = \frac{k_{s} \cdot (1 + b \cdot \theta_{c})}{\theta_{c} \cdot (\hat{\mu} - b) - 1} + [S_{i}]_{0}
$$

\n
$$
[X_{\tau}] = [X_{B}] + [X_{P}] + [X_{i}] = \frac{\theta_{c}}{\theta_{H}} \cdot \frac{Y \cdot ([S]_{0} - [S])}{1 + b \cdot \theta_{c}} + f \cdot b \cdot \theta_{c} \cdot [X_{B}] + [X_{i}]_{0} \cdot \frac{\theta_{c}}{\theta_{H}}
$$

\nNote that the fraction of the particular components that is made of active
\nbiomass decreases with increasing θ_{c} for the higher accumulation of $[X_{P}]$ e
\n $[X_{i}]$:

$$
\frac{[X_B]}{[X_T]} = \frac{\theta_C/\theta_H \cdot Y \cdot ([S]_0 - [S])}{(1 + f \cdot b \cdot \theta_C) \cdot \theta_C/\theta_H \cdot Y \cdot ([S]_0 - [S]) + [X_i]_0 \cdot (1 + b \cdot \theta_C)}
$$

The rate of sludge waste from the biological system, that equals the rate of accumulation of particulate matter, is calculated as the product of the waste flow rate Q_χ (=V/ $\mathsf{\theta}_\mathsf{C}$) per the particulate matter concentration in the system $[X_{\tau}]$. $(1 + f \cdot b \cdot V)$ *V*

$$
F_X = \frac{V}{\theta_C} \cdot [X_T] = Q \cdot Y \cdot ([S]_0 - [S]) \cdot \frac{(1+1 \cdot D \cdot V)}{1 + D \cdot \theta_C} + Q \cdot [X_i]_0
$$

The observed specific production of particulate organics referred to the amount of degraded organic matter, (observed yield: *Yobs*) is finally calculated as:

$$
Y_{obs} = \frac{F_X}{Q \cdot ([S]_0 + [X_i]_0 - [S])} = \frac{Y \cdot (1 + f \cdot b \cdot \theta_C)}{1 + b \cdot \theta_C} \cdot \frac{([S]_0 - [S])}{([S]_0 + [X_i]_0 - [S])} + \frac{[X_i]_0}{([S]_0 + [X_i]_0 - [S])}
$$

 $([S]_0 + [X_i]_0 - [S])$ $[X_i]$ $([S]_0 + [X_i]_0 - [S])$ $(1 + f \cdot b \cdot \theta_c)$ $([S]_0 - [S])$ $([S]_0 + [X_i]_0 - [S])$ $1 + b \cdot \theta_C$ $([S]_0 + [X_i]_0 - [S])$ $([S]_0 + [X_i]_0 - [S])$ *X* $[S]_0 + [X_i]_0 - [S]$ $S]_o - [S]$ *1 b* $Y \cdot (1 + f \cdot b)$ $Q \cdot ([S]_0 + [X_i]_0 - [S]$ *F Y* $_0$ \uparrow **i** \uparrow \uparrow j *i 0* $_0$ \top \blacksquare \wedge j j *0 C C* $_0$ \top \blacksquare \wedge j j *X obs*

The observed yield depends on:

- metabolic parameters of microorganisms that develop in the system:
	- \blacksquare it decreases with increasing decay rates (b);
	- It increases with increasing Y and f;
- The nature of the treated wastewater: specifically, it increases with decreasing WW degradability (degradable/total organics)
- Operational parameters: it decreases with increasing $\theta_{\textsf{C}}^{},$

Oxygen demand for organic matter oxidation $(F_{\Omega 2})$

It comprises 2 terms: an endogenous and an exogenous request:

The exogenous oxygen $(F_{O2,es})$ request is proportional to the organic matter that is degraded (equal to: $\mathsf{Q}(\mathsf{S}_0\text{-}\mathsf{S})$) multiplied by the fraction of it that is oxidized (equal to (1-Y)).

According to ASM1 process1:
$$
\frac{1}{Y_H}S_s + (\frac{1-Y_H}{Y_H})S_0 + i_{XB}S_{NH} + \frac{i_{XB}}{14}S_{ALK} \rightarrow X_{BH}
$$

it follows that: $\frac{\Delta S_0}{\Delta S_s} = (\frac{1-Y_H}{Y_H}) / \frac{1}{Y_H} = \langle -Y_H \rangle$
and therefore: $F_{O_2,ex} = Q \cdot ([S]_0 - [S]) \cdot \langle -Y_H \rangle$

Oxygen demand for organic matter oxidation (F_{O2})

The endogenous term $(F_{O2,end})$: it is proportional to the rate of production of soluble organic matter from biomass decay and following hydrolysis of the released X_S. rum
anc
dS

from biomass decay S S O O end the rate o f S releaseis proportiona l to the rate o f biomass decay: le t us assumethat betweenbiomass decayand hydrolysis, the first oneis the slowest d t S S F V 2 ,

Endogenous respiration according to ASM1

let us assume that between biomassdecay
\nlet us assume that between biomassdecay
\n
$$
\Rightarrow
$$
 the rate of S release is proportional to the rate of biomassdec
\n
$$
\left(\frac{dS_S}{dt}\right)_{from biomassdecay} = \left(\frac{dX_{BH}}{dt}\right)_{decay} \cdot \left(\frac{\Delta X_S}{\Delta X_{BH}}\right)_{decay} \cdot \left(\frac{\Delta S_S}{\Delta X_S}\right)_{hydrolysis}
$$
\nBy expressing each term according to the ASM1 model:
\n
$$
\Rightarrow \left(\frac{dS_S}{dt}\right)_{\text{total}} = b_H \cdot X_{BH} \cdot \left(-f_P \cdot \right)
$$

By expressing each term according to the ASM1m
\n
$$
\Rightarrow \left(\frac{dS_s}{dt}\right)_{from \text{biomassdecay}} = b_H \cdot X_{BH} \cdot \blacklozenge - f_P \cdot (1)
$$
\nand therefore: $F_{O_2, end} = V \cdot (1 - Y_H) \cdot b_H \cdot X_{BH} \cdot \blacklozenge - t$

 O_2 ,end $-V$ $V = I_H$ P $D_H \cdot D_H$ $R = I_P$ $\mathcal{V}_{2},$ end $= \mathsf{V} \cdot (1-\mathsf{Y}_H)$

Some comments on **biomass retention time**. Its value affects:

• the biomass concentration in the system

$$
[X_B] = \frac{\theta_C}{1 + b \cdot \theta_C} \cdot \frac{Y \cdot ([S]_0 - [S])}{\theta_H}
$$
 if $\theta_C \uparrow$ then $[X_B] \uparrow$

• the un-degraded soluble organic matter [S] that exit with the treated water and therefore the removal efficiency:

$$
\eta = 1 - [S]/[S]_0 = 1 - \frac{k_S \cdot (1 - b \cdot \theta_C)}{[S]_0 \cdot \theta_C \cdot (\hat{\mu} - b) - 1}
$$
 if $\theta_C \uparrow$ then $\eta \uparrow$

• the nature of the waste sludge:

$$
\frac{[X_{B}]}{[X_{T}]} = \frac{Y \cdot ([S]_{0} - [S])}{Y \cdot (1 + f \cdot b \cdot \theta_{C}) \cdot ([S]_{0} - [S]) + [X_{i}]_{0} \cdot (1 + b \cdot \theta_{C})}
$$
 if $\theta_{C} \uparrow$ then $[X_{B}]/[X_{T}] \uparrow$

Since $[X_{B}]$ is the most fermentable fraction of the whole sludge components if $\theta_{\rm C} \uparrow \rightarrow$ the sludge is more stabilized

Some comments on **biomass retention time**. Its value affects:

• The amount of waste sludge

$$
Y_{obs} = \frac{F_X}{Q \cdot ([S]_0 + [X_i]_0 - [S])} = \frac{Y \cdot (1 + f \cdot b \cdot \theta_C)}{1 + b \cdot \theta_C} \cdot \frac{([S]_0 - [S])}{([S]_0 + [X_i]_0 - [S])} + \frac{[X_i]_0}{([S]_0 + [X_i]_0 - [S])}
$$

if $\theta_C \uparrow$ then $Y_{obs} \uparrow$

• the oxygen request:

$$
F_{\text{O}_2} = F_{\text{O}_2,\text{end}} + F_{\text{O}_2,\text{ex}} = Q \cdot ([S]_0 - [S]) \cdot (-Y_H) + V \cdot (1 - Y_H) \cdot b_H \cdot X_{BH} \cdot (-I_B)
$$

if $\theta_C \uparrow$ then $F_{\text{O}_2} \uparrow$

Qualitative graph describing the dependence of the main system components from the biomass residence time:

• The biomass residence time should be sufficiently high to allow for biomass growth in the system. If microorganisms do not remain in the reactor at least for their duplication time, they would leave the system too early to give birth to a new generation \rightarrow microorganisms wash-out will inexorably occur.

• How long is this **minimum value** $\theta_{\text{C,min}}$?

 $P_C \rightarrow \theta_{C,min}$ \Rightarrow $[X_B] \rightarrow 0 \Rightarrow$ $[S] \rightarrow [S]_0$ *whash- out condition: if* $\theta_c \rightarrow \theta_{C,min}$ \Rightarrow $[X_B] \rightarrow 0 \Rightarrow$ $[S] \rightarrow [S]$

From eq.2
$$
\frac{1}{\theta_C} = \left[\hat{\mu} \cdot \left(\frac{[S]}{[S]+k_S}\right) - b\right]
$$

\nbut, for $\theta_C = \theta_{C,min}$ then $[S] = [S]_0$
\n $\frac{1}{\theta_{C,min}} = \left[\hat{\mu} \cdot \left(\frac{[S]_0}{[S]_0 + k_S}\right) - b\right]$
\nunder the assumption that : $[S]_0 \gg k_S$
\n $\frac{1}{\theta_C} = \hat{\mu} - b$

C ,min

Theevacuationrate the net growthrate

Simplified procedure for a basic design of the biological reactor:

Data:

- wastewater characteristics: Q, $[S]_0$, $[X]_0$, $[S_i]_0$, $[X_i]_0$
- $-$ design effluent concentrations : $[S]_T=[S]+[S]$
- maximum biomass concentration that is admissible (depending on the solid/separation unit): $[X]_{Tmax}$,

We want to calculate:

- the volume of the biological reactor,
- the amount of waste sludge that is produced and needs further treatment,
- the amount of oxygen that has to be dissolved by the aeration system

1. The minimum θ_c that is requested to achieve the desired $[S]_{T}$,

$$
[S_T] = \frac{k_{\rm s} \cdot (1 + b \cdot \theta_{\rm c})}{\theta_{\rm c} \cdot (\hat{\mu} - b) - 1} + [S_i]_0 \Rightarrow \theta_{\rm c} = \frac{k_{\rm s} + ([S]_T - [S_i]_0)}{([S]_T - [S_i]_0) \cdot (1 - b) - k_{\rm s} \cdot b}
$$

2. The minimum value of θ_H that corresponds to the maximum $[X]_T e$:

$$
[X_T] = [X_B] + [X_P] + [X_I] = \frac{\theta_C}{\theta_H} \cdot \frac{Y \cdot ([S]_0 - [S])}{1 + b \cdot \theta_C} (1 + f \cdot b \cdot \theta_C) + [X_I]_0 \cdot \frac{\theta_C}{\theta_H}
$$

$$
\theta_H = \frac{\theta_C}{[X]_T} \cdot \left[\frac{Y \cdot (S]_0 - [S]_T - [S_I]_0 \cdot \frac{S}{I} (1 + f \cdot b \cdot \theta_C)}{1 + b \cdot \theta_C} + [X_I]_0 \right]
$$

3. The reactor minimum reactor volume corresponding to the minimum hydraulic residence time:

4. The waste sludge production: $V = Q \cdot \theta_H$
The waste sludge pr
 $\begin{aligned} \nabla \cdot \mathbf{V} \cdot (\mathbf{[S]}_0 - \mathbf{[S]}) \n\end{aligned}$

$$
F_{X} = Q \frac{Y \cdot ([S]_{0} - [S])}{1 + b \cdot \theta_{C}} + f \cdot b \cdot V \cdot [X_{B}] + Q \cdot [X_{i}]_{0}
$$

5. The oxygen request:

$$
F_{\mathcal{O}_2} = Q \cdot ([S]_0 - [S]) \cdot \blacklozenge - Y_H + V \cdot (1 - Y_H) \cdot b_H \cdot X_{BH} \cdot \blacklozenge - f_P
$$

What if the reactor hydrodynamics is more similar to a plug flow reactor?

Along the PF reactor:

- soluble degradable organic matter has a decreasing gradient from the inlet to the outlet
- the concentration of un-degradable species does not change (S_i, X_i)
- Hyp: the concentration gradient of biomass X_B is negligible in most operational conditions (*Actual average concentration is two order of magnitude higher that the amount that grows across the reactor for biomass synthesis)*

This means that :

- the reactor behaviour is PF for [S]
- is like a CFSTR for all other components

Mass balances are therefore:

eq.1:
$$
\frac{d[S]}{r_C} = \frac{d[S]}{\left[-\frac{\hat{\mu}}{Y} \cdot \left(\frac{S}{[S] + k_S}\right) \cdot [X_B]\right]} = d\theta
$$

eq.2:
$$
0 = \frac{(0 - [X_B])}{\theta_C} + \left[\hat{\mu} \cdot \left(\frac{S}{[S] + k_S}\right) \cdot [X_B] - b \cdot [X_B]\right]
$$

eq.3:
$$
0 = \frac{(0 - [X_P])}{\theta_C} + \left[\mathbf{i} \cdot b \cdot [X_B] - 0\right]
$$

Biomass concentration is again:

 \mathbf{r} \sim

$$
[X_{B}] = \frac{\theta_{C}}{\theta_{H}} \cdot \frac{Y \cdot ([S]_{O} - [S]_{outPF})}{1 + b \cdot \theta_{C}}
$$

eq. 1 :

When integrating

 \cdot [X]_B is assumed as constant along the PF reactor;

•Inlet substrate concentration ($[S]_{in}$) results from the mixing of the inflow and the recycle flow :

$$
[\mathsf{SI}_{in} = [\mathsf{SI}_{0} \cdot \frac{1}{1+r} + [\mathsf{SI} \cdot \frac{r}{1+r}]
$$

 $\mathbf{r} \wedge$

• the actual residence time for the PF reactor is: θ_H '= θ_H /(1+r), where θ_H = V/Q as usual.

Therefore:

$$
\frac{d[S]}{r_c} = \frac{d[S]}{\left[-\frac{\hat{\mu}}{Y} \cdot \left(\frac{[S]}{[S] + k_S}\right) \cdot [X_B]\right]} = d\theta
$$

$$
-dS \cdot \left(\frac{\binom{[S] + k_S}{[S]}}{[S]} \right) = \frac{\hat{\mu}}{Y} \cdot [X_B] \cdot d\theta
$$

$$
-dS \cdot \left(\frac{|\mathbf{S}| + k_{S}}{[S]} \right) = \frac{\hat{\mu}}{Y} \cdot [X_{B}] \cdot d\theta
$$

\nBy integrating from the inletsection[S] = [S]_{in}, and $\theta = 0$
\nand the outletsection: [S] = [S]_{outPF}, and $\theta = \theta_{H}$:
\n([S]_{in} - [S]_{outPF}) + k_S \cdot ln \frac{[S]_{in}}{[S]_{outPF}} = \frac{\hat{\mu}}{Y} \cdot [X_{B}] \cdot \theta_{H}
\nby substituting: $[X_{B}] = \frac{\theta_{C}}{\theta_{H}} \cdot \frac{Y \cdot ([S]_{0} - [S]_{outPF})}{1 + b \cdot \theta_{C}}$
\nand: $\theta_{H} = \frac{\theta_{H}}{1 + r}$ and by solving for θ_{C}
\n
$$
\frac{1}{\theta_{C}} = \frac{\hat{\mu} \cdot ([S]_{0} - [S]_{outPF})}{(1 + r) \cdot [(S]_{in} - [S]_{outPF}) + k_{S} \cdot ln \frac{[S]_{in}}{[S]_{outPF}}]}
$$
 - b

[S]_{out PF}, can not be explicitly calculated, it depends on θ_{C} , but also from [S] $_0$ (unlike for CFSTR reactor). Generally, the outlet concentration is lower, the reactor is more efficient, but is also more sensitive to accidental toxicity in the influent.

They allow for the setting of:

- the hydraulic connections of various reactors
- for each reactor:
	- hydraulic behaviour
	- reactivity (Petersen Matrix):

They allow to:

•Create the set of differential equations that describe the dynamic response of the system •Integrate it numerically

Example: WEST (Worldwide Engine For Simulation, Training And Automation)

Output: dynamic simulation

Andamento della concentrazione di biomassa autotrofa

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